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## MEMORANDUM

| To: | Joshua DeVries, AICP - Osceola County |
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| From: | Mike Woodward, P.E. - Kimley-Horn and Associates, Inc. |
| Date: | August 14, 2019 |
| Subject: | Revised Traffic Analysis Methodology |
|  | Neptune Road PD\&E |
|  | PS-18-9905-DG |

Consistent with the scope of services for the Neptune Road Project Development and Environment (PD\&E) study, the traffic analysis will be conducted based on methods and procedures described in the Florida Department of Transportation (FDOT) PD\&E Manual, the FDOT Traffic Analysis Handbook, and the FDOT Project Traffic Forecasting (PTF) Handbook. This traffic analysis will be documented in the Project Traffic Analysis Report (PTAR).

This revised methodology addresses changes based on comments from FDOT. Upon agreement from the FDOT on this proposed methodology, Kimley-Horn will revise the PTAR as described herein.

A project description and transportation objectives are described in Appendix A.

## Proposed Study Area

The study area is illustrated in Figure 1 and consists of Neptune Road, from Partin Settlement Road to US 192, including the following intersections:

- Partin Settlement Road
- Cross Prairie Parkway
- Henry Partin Road
- Ames Haven Road
- Tohoqua Entrance / Neptune Middle School Entrance
- Sergeant Graham Drive / Commerce Center Drive
- Old Canoe Creek Road
- US 192


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Figure 1: Study Area


## Analysis Periods

Conditions will be analyzed for the following years:

- Existing Conditions: Year 2018
- Opening Year: 2025
- Design Year: 2045

If applicable (i.e., prior to 2045), an analysis of conditions at the year of exceeding capacity will be provided. Year of exceeding capacity is defined as the year when the arterial Level-of-Service (measured from Partin Settlement to US 192) falls to LOS F based on a SYNCHRO arterial LOS analysis.

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An analysis of No-Build conditions will be performed for each analysis year. Build Alternatives will be analyzed in the Opening and Design analysis years. AM and PM peak hour conditions will be analyzed for each scenario.

## Build Alternative

The build alternative will consist of widening Neptune Road to a 4-lane divided roadway with premium bicycle and pedestrian facilities (i.e., bike lanes, multiuse path(s), and/or sidewalks) where feasible. It is anticipated that various intersection alternatives will be considered in the analysis. Potential intersection improvements will be identified based on anticipated deficiencies, operational concerns, and public involvement comments. Alternative improvements (i.e., two eastbound lanes, a center turn lane, and one westbound lane) will be evaluated between Canoe Creek Road and US 192.

## Measures of Effectiveness

Measures of Effectiveness will include the following:

| Intersection | Arterial | Safety |
| :---: | :---: | :---: |
| Queue lengths | Travel Speed | Crash Frequency |
| Volume to Capacity Ratios | Arterial Level-of-Service | Crash Rate |
| Delay | -- | Number of Fatalities |
| Level-of-Service | -- | Crash Severity Level |
| -- | -- | Economic Loss |

While the County does not have LOS standards for transportation, they do consider V/C ratios as one of many factors (which are related to the County's mobility indicators) in establishing priorities for roadway improvements. It should be noted that while V/C ratios below 1.0 are desirable, they are not required by Osceola County.

Synchro software will be used to evaluate the arterial and study area intersections with the Measures of Effectiveness shown above. FDOT's Intersection Control Evaluation (ICE) Stage 1 screening will be performed using the Capacity Analysis for Planning of Junctions (CAP-X) for all study area intersections. The ICE findings will be screened based on context, practicality, right-of-way impacts, and cost. ICE stage 2 analysis will be conducted as applicable.

The analyses will also consider multimodal conditions, including facilities to accommodate pedestrians, bicyclists, transit, and freight. This will be accomplished by counting pedestrians and bicyclists during the turning movement counts, and vehicle classification counts to identify the percentage of trucks. Currently, there is no transit service on Neptune Road and no transit is planned for Neptune Road. Autonomous transit will also be considered as part of the PD\&E and the results of the PTAR will be considered in the review of potential autonomous transit service. The accommodation of pedestrians and bicyclists will be considered in the intersection analysis. Separate performance measures for bicyclists and pedestrians will not be evaluated.

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Access management strategies will be considered as they relate to safety and operational efficiency.

## Data Sources

Traffic counts previously provided by the County will be used for the analysis. The traffic counts were taken on Thursday, May 17, 2018. The seasonal factor for Osceola County was determined based on reports from FDOT. Historic traffic counts have been collected from County and FDOT sources. The Central Florida Regional Planning Model (CFRPM) version 6.1 will be used for modeling tasks. Transit ridership and origin/destination data, provided by LYNX, will be used to assess transit.

New ZDATA (to be provided by FDOT) will be compared to ZDATA from the adopted regional travel demand model to see if the new ZDATA has significant differences that may affect future traffic projections.

## Intersection Balancing

Intersection volumes (from counts) will be entered into the FDOT TURNS5 program to balance volumes. Manual adjustments may be performed as needed to better replicate actual turning patterns. This balancing step will be performed for each analysis scenario.

## Existing Conditions

Existing roadway segment data are provided in Table 1.

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Table 1: Existing Roadway Characteristics

| ROADWAY SEGMENT | $\begin{aligned} & 2018 \\ & \text { ADT } \end{aligned}$ | Average Peak Hour Volume |  | Peak Hour SB/WB | Measured K Factor | Measured D Factor | $\begin{gathered} \text { Axle } \\ \text { Adj. } \\ \text { Factor } \end{gathered}$ | Seasonal Adj. Factor | $2018$ AADT |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mainline |  |  |  |  |  |  |  |  |  |
| Neptune Road |  |  |  |  |  |  |  |  |  |
| West of Partin Settlement Rd | 35,634 | 2,743 | 1,811 | 932 | 7.7\% | 66\% | 0.99 | 1.01 | 36,000 |
| Partin Settlement Rd to Cross Prairie Pkwy | 25,825 | 2,615 | 1,088 | 1,527 | 10.1\% | 58\% | 0.99 | 1.01 | 26,000 |
| Cross Prairie Pkwy to Old Canoe Ck Rd | 24,394 | 1,998 | 768 | 1,230 | 8.2\% | 62\% | 0.99 | 1.01 | 24,000 |
| Old Canoe Ck Rd to US 19 | 21,336 | 1,233 | 456 | 777 | 5.8\% | 63\% | 0.99 | 1.01 | 21,000 |
| East of US 192 (Brown Chapel Rd) | 4,244 | 524 | 398 | 126 | 12.3\% | 76\% | 0.99 | 1.01 | 4,200 |
| Other Study Area Roadways |  |  |  |  |  |  |  |  |  |
| Partin Settlement Road |  |  |  |  |  |  |  |  |  |
| Neptune Rd to US 192 | 13,452 | 1,162 | 474 | 688 | 8.6\% | 59\% | 0.99 | 1.01 | 13,000 |
| Cross Prairie Parkway |  |  |  |  |  |  |  |  |  |
| South of Neptune Rd | 3,717 | 298 | 194 | 104 | 8.0\% | 65\% | 0.99 | 1.01 | 3,700 |
| Old Canoe Creek Road |  |  |  |  |  |  |  |  |  |
| US 192 to Neptune Rd | 19,212 | 1,515 | 681 | 834 | 7.9\% | 55\% | 0.99 | 1.01 | 19,000 |
| Neptune Rd to Kissimmee Park Rd | 32,050 | 2,049 | 1,398 | 651 | 6.4\% | 68\% | 0.99 | 1.01 | 32,000 |
| US 192 |  |  |  |  |  |  |  |  |  |
| Old Canoe Creek Rd to Neptune Rd | 37,428 | 2,541 | 1,316 | 1,225 | 6.8\% | 52\% | 0.99 | 1.01 | 37,000 |
| Neptune Rd to Columbia Ave | 42,056 | 2,849 | 1,365 | 1,484 | 6.8\% | 52\% | 0.99 | 1.01 | 42,000 |

Note - The 2018 ADT for Brown Chapel Road was determined by factoring the intersection turning movement counts. The ADT for all other segments was determined based on 72 -hour segment counts.

## Development of Future Volumes

Future volumes will be developed using methods described in the FDOT PTF Handbook.
The adopted future travel demand model, CFRPM version 6.1, was compared to a model developed by Central Florida Expressway Authority (CFX), which is a modified version of the CFRPM. The CFX model was developed for evaluating existing and future expressways in and around Osceola County and resulted in improved accuracies for these types of regional roadways. However, the traffic forecasts on roadways in the study area were more accurate in the CFRPM than in the CFX model. Table 2 shows a comparison of base model conditions compared to actual volumes for study area

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segments. Note that segments along Neptune Road are within $15 \%$ of actual volumes in the CFRPM model, but over 70\% too low in the CFX model. Side-street segments also perform significantly better in the CFRPM than in CFX, where CFX volumes are too high by $66 \%$ and CFRPM volumes are too low by $26 \%$.

Table 2: Comparison of CFX base model and CFRPM base model to actual volumes

| ROADWAY SEGMENT | CFX: <br> $\mathbf{2 0 1 5}$ | Actual <br> $\mathbf{2 0 1 5}$ | $\mathbf{2 0 1 5} \mathbf{2 0 1 5}$ <br> Actual | Percent <br> Deviation | CFRPM: <br> $\mathbf{2 0 1 0}$ | Actual 2010 <br> CFRPM / <br> $\mathbf{2 0 1 0}$ <br> Actual | Percent <br> Deviation |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Standard |  |  |  |  |  |  |  |

Further, future population and employment data within the CFX model were lower than the mediumhigh projections within the CFRPM. Population and employment data within the CFX model were particularly low in the South Lake Toho area, east of Lake Tohopekaliga, and near Neptune Road. Due to these factors, the CFRPM was selected as the most appropriate model to use.

After the initial comparison of models, a sub-area model validation process was performed for the CFRPM by comparing model performance for Year 2018 to actual volumes from Year 2018. This effort was intended to determine how well the travel demand model performs on roadways within the study area. The validation process is described in greater detail in subsequent sections.

The future year models within the CFRPM do not include Cross Prairie Parkway, so the facility was added as a four-lane divided arterial. Centroid connectors near Cross Prairie Parkway were modified based on this network change, with zones west of the Turnpike generally connecting to Cross Prairie Parkway rather than to the network east of the Turnpike. This modification does not affect the base model since the facility was not in place in Year 2010 (which is the base year for CFRPM 6.1). For reference, model network printouts for Year 2045 are provided in Appendix B, with and without Cross Prairie Parkway in place.

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## Historic Traffic Data

Historic traffic volume trends on study area roadways were identified using data from Osceola County's Traffic Count program and data from FDOT's Florida Traffic Information (FTI) database. Historic growth rates on study area roadways are shown in Table 2. Historic growth data are provided in Appendix C.

Table 2: Historic Growth Rates

| ROADWAY SEGMENT | 2008 | 2009 | 2010 | 2011 | 2012 | 2013 | 2014 | 2015 | 2016 | 2017 | 2018 | Historic Growth Rate |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mainline |  |  |  |  |  |  |  |  |  |  |  |  |
| Neptune Road |  |  |  |  |  |  |  |  |  |  |  |  |
| West of Partin Settlement Rd |  |  | 19,932 |  | 23,102 |  |  | 25,943 | 24,286 | 31,370 | 29,670 | 4.08\% |
| Partin Settlement Rd to Cross Prairie Pkwy |  |  | 16,234 |  | 18,773 |  |  | 21,553 | 23,378 | 20,286 |  | 3.56\% |
| Cross Prairie Pkwy to Old Canoe Ck Rd |  |  | 16,234 |  | 18,773 |  |  | 21,553 | 23,378 | 20,286 |  | 3.56\% |
| Old Canoe Ck Rd to US $192$ | 8,490 | 8,520 | 8,249 |  | 8,763 |  |  | 11,252 | 11,922 | 12,287 |  | 4.05\% |
| East of US 192 (Brown Chapel Rd) |  | 5,873 |  |  | 6,658 |  |  | 7,142 | 8,059 | 8,568 | 8,686 | 3.70\% |
| Other Study Area Roadways |  |  |  |  |  |  |  |  |  |  |  |  |
| Partin Settlement Road |  |  |  |  |  |  |  |  |  |  |  |  |
| Neptune Rd to US 192 | 11,180 | 10,937 | 9,961 |  | 10,911 |  |  | 12,750 | 12,959 | 13,628 | 14,119 | 2.68\% |
| Old Canoe Creek Road |  |  |  |  |  |  |  |  |  |  |  |  |
| US 192 to Neptune Rd | 16,553 | 16,712 | 16,308 |  | 18,430 |  |  | 19,242 | 20,382 | 18,298 | 19,536 | 1.71\% |
| Neptune Rd to <br> Kissimmee Park Rd | 24,311 |  | 24,343 |  | 25,551 |  |  | 23,747 | 26,251 | 26,053 | 24,807 | 44.00\% |
| US 192 |  |  |  |  |  |  |  |  |  |  |  |  |
| Old Canoe Creek Rd to Neptune Rd |  |  |  |  |  | 38,000 | 40,000 | 36,500 | 37,500 | 39,500 |  | 0.13\% |
| Neptune Rd to Columbia Ave |  |  |  |  |  | 40,000 | 41,000 | 45,000 | 45,000 | 47,000 |  | 3.81\% |

## Population Projections

Population growth forecasts for Osceola County were identified from the Bureau of Economic and Business Research (BEBR), as shown in Table 3. The average of the medium and high growth rates between 2017 and 2045 is 2.95\%

Table 3: BEBR Population Forecast for Osceola County

| Estimate | 2017 BEBR <br> Population | 2025 BEBR <br> Projected <br> Population | 2045 BEBR <br> Projected <br> Population | Growth Rate <br> $2017-2025$ | Growth Rate <br> $2017-2045$ |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Low | 337,614 | 400,000 | 495,700 | $2.3 \%$ | $1.4 \%$ |
| Medium | 337,614 | 452,400 | 649,800 | $4.2 \%$ | $2.5 \%$ |
| High | 337,614 | 495,500 | 808,100 | $5.8 \%$ | $3.4 \%$ |

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## Sub-Area Model Validation

A sub-area model validation analysis was performed to improve the accuracy of the travel demand model within the study area. Existing year (2018) AADT values for roadway segments were obtained from the volume and classification counts performed for this project as well as the most recent Osceola County Traffic Count Reports. Model AADTs for year 2018 were derived by interpolating model volumes from the base year (2010) and interim year (2020), then multiplying the output volume by the Osceola County Model Output Conversion Factor (MOCF). The MOCF is identified on the FDOT FTI seasonal factor sheet. The Year 2020 model network was modified to include the existing laneage for US 192, which is in the process of being widened to a 6-lane facility. Since it was a 4-lane roadway in Year 2018, the model was modified to reduce the laneage from 6-lanes to 4-lanes. This step was necessary for the comparison of 2018 actual volumes (where US 192 is a 4-lane road) to 2018 interpolated model volumes. Similarly, the Year 2020 model was modified to reduce laneage on Neptune Road to match existing laneage. Specifically, the model was modified from showing Neptune Road as a 4-lane facility to including it as a 2-lane facility. The Facility Type was changed from 32 to 23 to improve the model accuracy.

The model population for Year 2020 was compared to the average of the BEBR medium and high population projections for Year 2020 and was found to be approximately $1.3 \%$ higher than the BEBR average. Due to the similar populations, there are no recommended ZDATA edits or population factors.

Initial model runs indicated that model volumes were unreasonably high on Brown Chapel Road, northeast of US 192. The facility type and area type factors were therefore modified to more accurately represent the roadway and this resulted in more accurate results. These changes were also made to all future years for this segment. The facility type was changed from 43 to 46.

As identified in the FDOT PTF Handbook, the three measures of effectiveness for the model accuracy assessment are as follows:

- the model volume-to-count ratio for study area links
- the model volume-to-count ratio for cutlines
- the Root Mean Square Error (RMSE)

As stated in the FDOT PTF Handbook, the acceptable volume-to-count ratio for arterials is $\pm 15 \%$. For collector roads, a larger difference of $\pm 25 \%$ is acceptable. Model volumes interpolated to Year 2018 were compared to count data, as shown in Table 4.

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Table 4: Comparison of Model Volumes to Actual Traffic Count Data

| ROADWAY SEGMENT | $\begin{gathered} 2018 \\ \text { AADT } \end{gathered}$ | Interp. 2018 Model ADT | $\begin{aligned} & \text { Factored } \\ & 2018 \text { Model } \\ & \text { AADT } \end{aligned}$ | 2018 CFRPM AADT / 2018 AADT | Percent Deviation | Standard |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mainline Characteristics |  |  |  |  |  |  |
| Neptune Road |  |  |  |  |  |  |
| West of Partin Settlement Rd | 36,000 | 27,775 | 27,000 | 0.75 | -25\% | +/-15\% |
| Partin Settlement Rd to Cross Prairie Pkwy | 26,000 | 19,627 | 19,000 | 0.73 | -27\% | +/-15\% |
| Cross Prairie Pkwy to Old Canoe Ck Rd | 24,000 | 19,921 | 19,000 | 0.79 | -21\% | +/-15\% |
| Old Canoe Ck Rd to US 192 | 21,000 | 14,639 | 14,000 | 0.67 | -33\% | +/-15\% |
| East of US 192 (Brown Chapel Rd) | 4,200 | 8,651 | 8,400 | 2.00 | 100\% | +/- 25\% |
| Side Street Characteristics |  |  |  |  |  |  |
| Partin Settlement Road |  |  |  |  |  |  |
| Neptune Rd to US 192 | 13,000 | 9,233 | 9,000 | 0.69 | -31\% | +/- $25 \%$ |
| East/North of US 192 | 14,000 | 11,995 | 12,000 | 0.86 | -14\% | +/- $25 \%$ |
| Old Canoe Creek Road |  |  |  |  |  |  |
| US 192 to Neptune Rd | 19,000 | 19,816 | 19,000 | 1.00 | 0\% | +/-15\% |
| Neptune Rd to Kissimmee Park Rd | 32,000 | 30,879 | 30,000 | 0.94 | -6\% | +/-15\% |
| US 192 |  |  |  |  |  |  |
| Shady Ln to Partin Settlement Rd | 37,000 | 52,694 | 51,000 | 1.38 | 38\% | +/-15\% |
| Old Canoe Creek Rd to Neptune Rd | 37,000 | 30,139 | 29,000 | 0.78 | -22\% | +/-15\% |
| Neptune Rd to Columbia Ave | 42,000 | 39,482 | 38,000 | 0.90 | -10\% | +/-15\% |
| Total | 305,200 |  | 275,400 | 0.902 | -9.76\% | +/-5\% |

As shown in Table 4, Neptune Road and the side streets have model volumes that are much lower than actual volumes. Few individual segments fit within the standard threshold for error, and the total area-wide error exceeds the standard error.

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The acceptable level of accuracy for screenlines/cutlines is based on the AADT:

- Cut-lines with greater than 70,000 AADT: $\pm 10 \%$
- Cut-lines with 35,000 to 70,000 AADT: $\pm 15 \%$
- Cut-lines with less than 35,000 AADT: $\pm 20 \%$

The screenline comparison is shown in Table 5. As shown in the table, both cutlines exceed the error threshold.

Table 5: Model Cutlines

| Cutline Number | Road | Location | AADT (Count) | AADT (Model) | Difference | Standard |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Cutline 1 | Neptune Road US 192 | West of Partin Settlement Rd East of Shady Ln Total Cutline 1 | $\begin{array}{r} 36,000 \\ 37,000 \\ 73,000 \\ \hline \end{array}$ | $\begin{aligned} & 27,775 \\ & 52,694 \\ & 80,469 \\ & \hline \end{aligned}$ | $\begin{gathered} -22.8 \% \\ 42.4 \% \\ 10.2 \% \end{gathered}$ | $+/-10 \%$ |
| Cutline 2 | Neptune Road <br> US 192 | East of Old Canoe <br> Creek Rd <br> East of Old Canoe <br> Creek Rd <br> Total Cutline 2 | $\begin{array}{r} 21,000 \\ 37,000 \\ 58,000 \\ \hline \end{array}$ | $\begin{aligned} & 14,639 \\ & 29,000 \\ & 43,639 \\ & \hline \end{aligned}$ | $\begin{aligned} & -30.3 \% \\ & -21.6 \% \\ & -24.8 \% \end{aligned}$ | $+/-15 \%$ |

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The Percent Root Mean Square Error (RMSE) for an area can be $\pm 35 \%$ to $45 \%$. The RMSE calculations are shown in Table 6. All volume groups fall within the acceptable errors, and the overall error is at the limit of being acceptable.

Table 6: RMSE Calculations

| Volume Group | Count AADT | Model AADT | RMSE | Standard |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  | Preferred | Acceptable |
| 5,000-9,999 | 0 | 0 | n/a | 35\% | 45\% |
| 10,000-14,999 | 27,000 | 21,000 | 23\% | 27\% | 35\% |
| 15,000-19,999 | 19,000 | 19,000 | 0\% | 25\% | 30\% |
| 20,000-29,999 | 71,000 | 52,000 | 27\% | 15\% | 37\% |
| 30,000-49,999 | 263,000 | 244,000 | 22\% | 15\% | 25\% |
| 50,000-59,999 | 0 | 0 | n/a | 10\% | 20\% |
| Over 60,000 | 0 | 0 | n/a | 10\% | 19\% |
| RMSE Areawide | 380,000 | 336,000 | 45\% | 35\% | 45\% |

As shown in Tables 4, 5, and 6, the individual segments, overall area, and cutlines have model volumes that exceed the standard errors allowed per the FDOT PTF Handbook. The overall RMSE calculations adequately represent existing conditions, with the area-wide measure at the acceptable standard. Due to the overall model performance in the area, it is recommended that the travel demand model be used with caution, and only as needed, when forecasting future volumes. For existing segments, model growth rates should be considered in conjunction with historic growth and population projections. For new roadways (i.e., Cross Prairie Parkway), the travel demand model should be used to forecast future volumes.

## Model Growth Rates

Model growth rates are calculated by adding the growth between the projected 2018 volumes and the projected 2045 volumes to the measured 2018 volumes, then determining the growth rate. This assumes that the model provides a reasonable estimate of future traffic growth, even if it does not accurately project the 2018 conditions. The Year 2045 model was run under No-Build conditions with Neptune Road as a 2-lane facility between Partin Settlement Road and US 192. For Build conditions, a Year 2045 model was run with Neptune Road as a 4-lane divided roadway from Partin Settlement Road to US 192. Model growth rates for Build and No-Build conditions are shown in Table 7. The rates are based on model growth between model volumes interpolated to Year 2018 and the future Build and No-Build model volumes for Year 2045.

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Table 7: Future Model Volumes

| ROADW AY SEGMENT | $\begin{gathered} 2018 \\ \text { AADT } \end{gathered}$ | 2018 <br> Model AADT | No-Build |  |  | Build |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | $2045$ <br> Model AADT | Model Growth | Growth <br> Rate | 2045 <br> Model AADT | Model Growth | Growth Rate |
| Mainline Characteristics |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |
| West of Partin <br> Settlement Rd | 36,000 | 27,000 | 45,000 | 18,000 | 1.85\% | 46,000 | 19,000 | 1.95\% |
| Partin Settlement Rd to Cross Prairie Pkwy | 26,000 | 19,000 | 22,000 | 3,000 | 0.43\% | 36,000 | 17,000 | 2.42\% |
| Cross Prairie Pkwy to Old Canoe Ck Rd | 24,000 | 19,000 | 16,000 | -3,000 | -0.46\% | 29,000 | 10,000 | 1.54\% |
| Old Canoe Ck Rd to US 192 | 21,000 | 14,000 | 11,000 | -3,000 | -0.53\% | 18,000 | 4,000 | 0.71\% |
| Weighted A verage: |  |  |  |  | 0.85\% |  |  | 1.82\% |
| Side Street Characteristics |  |  |  |  |  |  |  |  |
| Partin Settlement Road Neptune Rd to US 192 | 13,000 | 9,000 | 29,000 | 20,000 | 5.70\% | 21,000 | 12,000 | 3.42\% |
| Old Canoe Creek Road |  |  |  |  |  |  |  |  |
| US 192 to Neptune Rd | 19,000 | 19,000 | 26,000 | 7,000 | 1.36\% | 22,000 | 3,000 | 0.58\% |
| Neptune Rd to Kissimmee Park Rd | 32,000 | 30,000 | 39,000 | 9,000 | 1.04\% | 42,000 | 12,000 | 1.39\% |
| Weighted A verage: |  |  |  |  | 1.17\% |  |  | 1.11\% |
| US 192 |  |  |  |  |  |  |  |  |
| Old Canoe Creek Rd to Neptune Rd | 37,000 | 29,000 | 41,000 | 12,000 | 1.20\% | 37,000 | 8,000 | 0.80\% |
| Neptune Rd to Columbia Ave | 42,000 | 38,000 | 48,000 | 10,000 | 0.88\% | 50,000 | 12,000 | 1.06\% |
| Weighted A verage: |  |  |  |  | 1.03\% |  |  | 0.95\% |

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## Growth Rate Recommendation

The historic growth rates, population forecasts, model growth rates, and recommended growth rates are shown in Table 8. Model volumes are recommended for Cross Prairie Parkway since the roadway is new and there is not a sufficient amount of existing or historic data.

Table 8: Growth Rate Comparison and Selection

| ROADWAY SEGMENT | CFRPM NoBuild Growth Rate | CFRPM <br> Build Growth Rate | Historical Trend Growth Rate | BEBR Growth Rate (Avg of Medium \& High) | No Build Growth Rate To Use | Source | Build <br> Growth <br> Rate To Use | Source |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Mainline Characteristics |  |  |  |  |  |  |  |  |
| Neptune Road |  |  |  |  |  |  |  |  |
| West of Partin Settlement Rd | 1.85\% | 1.95\% | 4.08\% | 2.95\% | 2.0\% | Blend of <br>  <br> Population Growth | 2.0\% | Blend of Model \& Population Growth |
| Partin Settlement Rd to Cross Prairie Pkwy | 0.43\% | 2.42\% | 3.56\% | 2.95\% | 1.0\% |  | 2.5\% |  |
| Cross Prairie Pkwy to Old Canoe Ck Rd | -0.46\% | 1.54\% | 3.56\% | 2.95\% | 1.0\% |  | 1.5\% |  |
| Old Canoe Ck Rd to US 192 | -0.53\% | 0.71\% | 4.05\% | 2.95\% | 1.0\% |  | 1.0\% |  |
| East of US 192 (Brown Chapel Rd) | -0.62\% | -0.88\% | 3.70\% | 2.95\% | 1.0\% |  | 1.0\% |  |
| Side Street Characteristics |  |  |  |  |  |  |  |  |
| Partin Settlement Road |  |  |  |  |  |  |  |  |
| Neptune Rd to US 192 | 5.70\% | 3.42\% | 2.68\% | 2.95\% | 5.0\% | Blend of Model \& Population | 3.0\% | BEBR |
| Cross Prairie Parkway |  |  |  |  |  |  |  |  |
| South of Neptune Rd | n/a | n/a | n/a | 2.95\% | Use Model Volumes |  |  |  |
| Old Canoe Creek Road |  |  |  |  |  |  |  |  |
| US 192 to Neptune Rd | 1.36\% | 0.58\% | 1.71\% | 2.95\% | 1.0\% | Minumum | 1.0\% | Minumum |
| Neptune Rd to Kissimmee Park Rd | 1.04\% | 1.39\% | 44.00\% | 2.95\% | 1.0\% | Minumum | 1.0\% | Minumum |
| US 192 |  |  |  |  |  |  |  |  |
| Old Canoe Creek Rd to Neptune Rd | 1.20\% | 0.80\% | 0.13\% | 2.95\% | 1.0\% | Minumum | 1.0\% | Minumum |
| Neptune Rd to Columbia Ave | 0.88\% | 1.06\% | 3.81\% | 2.95\% | 1.0\% | Minimum | 1.0\% | Minimum |

## Development of Design Traffic Characteristics

Design traffic characteristics will be developed in accordance with the Project Traffic Forecasting (PTF) Handbook, January 2014. The primary design traffic characteristics are the standard K factor, Design Hour Directional Demand (D) factor, and percentage of trucks for both the design hour and daily conditions ( $\mathrm{T}_{\mathrm{f},} \mathrm{T}_{24}$ ). These characteristics are used in developing the future traffic volumes and conducting future operational analyses.

The K factor defines the proportion between the design hour volume (DHV) and daily volume. As explained in the PTF Handbook, the K factor "defines the volume of traffic for which the road is

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designed to handle." FDOT has adopted a standard K factor to use in analyses based on area and facility type. For arterials and highway within an urbanized area, the standard K factor is $9.0 \%$. Therefore, a K factor of $9.0 \%$ (for PM peak hour) will be used for the study area roadways to develop DDHV representing the PM peak-hour conditions.

In addition to the analysis of DDHV conditions, it is desired to provide an AM peak-hour analysis. Therefore, existing traffic counts along Neptune Road were evaluated to develop an AM peak-hour peak-to-daily ratio. Based on these counts, a $7.0 \%$ factor is appropriate for calculating AM peak hour volumes. Therefore, AM peak hour segment volumes will be calculated by applying a factor of $7.0 \%$ to the daily volume. Additional information documenting the development of the $7 \%$ value is provided in Appendix D.

The $D$ factor is used to determine the directional split of traffic during the design hour. The D values for Neptune Road were obtained from roadway traffic counts. The D factors fall within the recommended range for similar types of roadways. FDOT's PTF Handbook recommends a range of D values based on facility type. A summary of the recommended values for an Urban Arterial are included in Table 9 below.

Table 9: Recommended Range of $D$ Values

| Facility Type | FDOT D Values |  |  |
| :---: | :---: | :---: | :---: |
|  | Low | Average | High |
| Urban Arterial | $50.80 \%$ | $57.90 \%$ | $67.10 \%$ |

It is recommended that the average D value of $57.9 \%$ be used for all segments in the study area. In the AM peak hour, the peak direction is westbound for Neptune Road and northbound for side streets. In the PM peak hour, the peak direction is eastbound for Neptune Road and southbound for side streets. The only exception is Ames Haven Road, where the AM peak direction is southbound, and the PM peak direction is northbound.

## $\mathrm{T}_{24}$ and $\mathrm{T}_{\mathrm{f}}$ Factors

Truck percentages were calculated for both daily ( $\mathrm{T}_{24}$ ) and peak hour ( $\mathrm{T}_{\mathrm{f}}$ ) conditions. Historical $\mathrm{T}_{24}$ values from count site 928063 of FDOT's FTI 2017 are listed in Table 10. As presented in the table, the historical $\mathrm{T}_{24}$ values range from $4.1 \%$ to $5.8 \%$, with an average value of $5.11 \%$.

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Table 10: Neptune Road Historical $T_{24}$ Values

| YEAR | $\mathrm{T}_{24}$ Factor |
| :---: | :---: |
| SITE 92-8063 |  |
| NEPTUNE RD, E OF OLD CANOE CK RD |  |
| 2011 | $5.4 \%$ |
| 2012 | $5.8 \%$ |
| 2013 | $5.3 \%$ |
| 2014 | $5.0 \%$ |
| 2015 | $4.4 \%$ |
| 2016 | $5.8 \%$ |
| 2017 | $4.1 \%$ |
| Average: | $5.11 \%$ |
| Minimum: | $4.10 \%$ |
| Maximum: | $5.80 \%$ |

As explained in the PTF Handbook, $T_{f}$ is estimated to equal at least half of $T_{24}$. Historical $T_{24}$ values were used to calculate $\mathrm{T}_{f}$ as shown in Table 11 below.

Table 11: Neptune Road Historical $T_{f}$ Values

| Measure | Tf Factor |
| :---: | :---: |
| Average: | $2.56 \%$ |
| Minimum: | $2.05 \%$ |
| Maximum: | $2.90 \%$ |

The average truck factors of $5.11 \%$ for $\mathrm{T}_{24}$ and $2.56 \%$ for $\mathrm{T}_{f}$ were used in the analysis. This is consistent with the recommendation from the PTF Handbook that the $T_{f}$ value should be at least half of the $\mathrm{T}_{24}$.

## Recommended Design Traffic Characteristics

The recommended design characteristics for this study are identified in Table 12. These are based on a review of historical and measured design traffic characteristics.

Table 12: Recommended Design Traffic Characteristics

| ROADWAY SEGMENT | K Factor <br> (PM) | AM Peak to <br> Daily Factor | D Factor | $\boldsymbol{T}_{\mathbf{2 4}}$ Factor | $\boldsymbol{T}_{\mathbf{f}}$ Factor |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Neptune Road | $9.0 \%$ | $7.0 \%$ | $57.9 \%$ | $5.1 \%$ | $2.6 \%$ |

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## Safety Analysis

A safety analysis will be performed by documenting historic crash trends. Five years of data, from $1 / 1 / 2013-12 / 31 / 2017$, will be summarized in terms of the total crashes, injury crashes, fatal crashes, property damage, crashes in the dark or on wet surfaces, and crashes involving bicycles or pedestrians. Crashes will then be summarized by time of day and day of week. A crash type analysis will be performed to understand trends. A crash location analysis will be performed to identify locations that should be considered further.

The safety performance of the Build alternatives will be identified using Crash Modification Factors (CMFs) from the Federal Highway Administration (FHWA) clearinghouse.

Documentation of the safety analysis will be included within the PTAR.

## Multimodal Analysis

A multimodal analysis will be performed to document the presence, width, and location of facilities for walking, biking, and transit. Bicycle and pedestrian count data will be provided at study area intersections.

## Documentation

Results of the traffic analysis will be documented in a Project Traffic Analysis Report (PTAR). An Executive Summary will be provided. The PTAR will detail the intersection analysis and anticipated operational performance of study area roadways based on the performance measures described in this methodology. The anticipated Equivalent Single Axle Load (ESAL) calculations will be included in the PTAR. The PTAR will be delivered electronically as a PDF. Word, Excel, and Synchro files will also be provided to the County.

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## Appendix A Description of Purpose

## Project Desaription

This project involves a 3.9-mile segment of Neptune Road extending from Partin Settlement Road to US 192 in Osceola County. The section east of the St. Cloud canal (approximately 1.1 miles in length) is within the City of St. Cloud. From Partin Settlement Road to Old Canoe Creek Road, the proposed project improves the existing 2-lane roadway to a 4-lane, divided roadway with a curbed median, with premium bicycle and pedestrian facilities (i.e., bike lanes, multiuse path(s), and/or sidewalks). From Old Canoe Creek Road to US 192, the project widens the existing 2-lane roadway to 4-lanes with sidewalks. Bridge structures are to be replaced and stormwater management facilities will be evaluated. Figure 1 illustrates the project location.

Figure 1: Project Location


## Traffic Analysis Objective

The primary purpose of improving Neptune Road is to enhance mobility from US 192 and St. Cloud to Downtown Kissimmee, improve access to NeoCity, and improve overall traffic operations of the existing highway network within the project study area. The secondary objectives are to provide transportation infrastructure to support economic growth, provide consistency with local plans and policies, and enhance safety.

The need for the project is to provide system linkage, provide additional capacity, address transportation demand, meet social and economic needs, provide improved modal interrelationships, improve safety and achieve consistency with transportation plans.

The objective of the traffic analysis is to document the performance of alternatives in providing additional capacity to address the future transportation demand and to improve safety.

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## Year 2045 Model Network Printouts




C:|FSUTMS\D5\CFRPMV61_Daily CBaselCF_2045 Modifified

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## Appendix C Historic Growth Trend Worksheets

Traffic Trends - V2.0
NEPTUNE RD -- West of Partin Settlement Rd


| County: | Osceola (92) |
| :---: | :---: |
| Station \#: | 0 |
| Highway: | NEPTUNE RD |





Traffic Trends - V2.0
NEPTUNE RD -- Partin Settlement to Cross Prairie


| County: | Osceola (92) |
| :---: | :---: |
| Station \#: | 0 |
| Highway: | NEPTUNE RD |




| Year | Traffic (ADT/AADT) |  |
| :---: | :---: | :---: |
|  | Count* | Trend** |
| 2010 | 16200 | 17000 |
| 2011 | 17500 | 17800 |
| 2012 | 18800 | 18600 |
| 2013 | 19700 | 19400 |
| 2014 | 20600 | 20200 |
| 2015 | 21600 | 21000 |
| 2016 | 23400 | 21800 |
| 2017 | 20300 | 22600 |
|  |  |  |
|  |  |  |
|  |  |  |
|  |  |  |
|  |  |  |
|  |  |  |
| 2018 Opening Year Trend |  |  |
| 2018 | N/A | 23400 |
| 2025 Mid-Year Trend |  |  |
| 2025 | N/A | 29000 |
| 2045 Design Year Trend |  |  |
| 2045 | N/A | 45100 |
| TRANPLAN Forecasts/Trends |  |  |
|  |  |  |
|  |  |  |

Traffic Trends - V2.0


| County: | Osceola (92) |
| :---: | :---: |
| Station \#: | 0 |
| Highway: | NEPTUNE RD |





Traffic Trends - V2.0
NEPTUNE RD -- Old Canoe creek to US 192


| County: | Osceola (92) |
| :---: | :---: |
| Station \#: | 0 |
| Highway: | NEPTUNE RD |



| Year | Traffic (ADT/AADT) |  |
| :---: | :---: | :---: |
|  | Count* | Trend** |
| 2008 | 8500 | 7600 |
| 2009 | 8500 | 8100 |
| 2010 | 8200 | 8600 |
| 2011 | 8500 | 9100 |
| 2012 | 8800 | 9600 |
| 2013 | 9600 | 10000 |
| 2014 | 10400 | 10500 |
| 2015 | 11300 | 11000 |
| 2016 | 11900 | 11500 |
| 2017 | 12300 | 12000 |
|  |  |  |
|  |  |  |
|  |  |  |
|  |  |  |
|  |  |  |
| 2018 Opening Year Trend |  |  |
| 2018 | N/A | 12500 |
| 2025 Mid-Year Trend |  |  |
| 2025 | N/A | 15900 |
| 2045 Design Year Trend |  |  |
| 2045 | N/A | 25600 |
| TRANPLAN Forecasts/Trends |  |  |
|  |  |  |
|  |  |  |

Traffic Trends - V2.0
NEPTUNE RD -- North of US 192


| County: | Osceola (92) |
| :---: | :---: |
| Station \#: | 0 |
| Highway: | NEPTUNE RD |




Traffic Trends - V2.0
PARTIN SETTLEMENT RD -- Neptune to US 192

| PIN\# | 12345 |
| :--- | ---: |
| Location | 1 |


| County: | Osceola (92) |
| :---: | :---: |
| Station \#: | 0 |
| Highway: | PARTIN SETTLEMENT RD |




Traffic Trends - V2.0


| County: | Osceola (92) |
| :---: | :---: |
| Station \#: | 0 |
| Highway: | OLD CANOE CREEK RD |




Straight Line Growth Option
*Axle-Adjusted

Traffic Trends - V2.0
old canoe creek rd -- Neptune Rd to Kissimmee Park Rd


| County: | Osceola (92) |
| :---: | :---: |
| Station \#: | 0 |
| Highway: | OLD CANOE CREEK RD |



*Axle-Adjusted

Traffic Trends - V2.0
US 192 -- Commerce Center to Neptune Rd


| County: | Osceola (92) |
| :---: | :---: |
| Station \#: | 0 |
| Highway: | US 192 |




Straight Line Growth Option
*Axle-Adjusted

Traffic Trends - V2.0
US 192 -- Neptune Rd to Columbia Ave


| County: | Osceola (92) |
| :---: | :---: |
| Station \#: | 0 |
| Highway: | US 192 |




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## Appendix D

## Development of AM Peak-to-Daily Ratio

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## MEMORANDUM

To: Victor Muchuruza, P.E.
From: Mike Woodward, P.E.
Kimley-Horn and Associates, Inc.
Date: August 12, 2019
Subject: Development of AM Peak Hour Turning Movements, Neptune Road PD\&E

Multiple traffic counts were collected for the Neptune Road Project Development and Environment (PD\&E) study. This memorandum is provided to support the use of 7.0 percent of the daily traffic for the development of AM peak hour turning movements. This is not the K-Factor. A standard 9.0 percent K-Factor will be used to develop the design traffic, which corresponds with the PM peak hour conditions.

Of the traffic counts taken on the portion of Neptune Road to be improved, the segment from Partin Settlement Road to Cross Prairie Parkway has the highest volume. This is expected to be consistent for future conditions as well.

Based on a 72 -hour traffic count on Neptune Road, east of Partin Settlement Road, the PM peak hour represents the design hour for the roadway, accounting for between 9.1 percent and 9.9 percent of the daily traffic volume. The peak hour factors ranged from 0.89 to 0.90 , suggesting no peak spreading. A copy of this count is included in Attachment A. The hourly counts by day are summarized in Table 1 and the 15-minute counts used to identify the PM peak hour are identified in Table 2.

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Table 1: Daily Traffic Count, Neptune Road, East of Partin Settlement Road

| Ending <br> Hour | Tues <br> $\mathbf{5 / 8 / 2 0 1 8}$ | Wed <br> $\mathbf{5 / 9 / 2 0 1 8}$ | Thurs <br> $\mathbf{5 / 1 0 / 2 0 1 8}$ |
| :---: | :---: | :---: | :---: |
| 1 | 214 | 199 | 215 |
| 2 | 100 | 91 | 134 |
| 3 | 91 | 103 | 88 |
| 4 | 87 | 102 | 107 |
| 5 | 209 | 211 | 213 |
| 6 | 538 | 531 | 510 |
| 7 | 1,478 | 1,436 | 1,372 |
| 8 | 1,728 | 1,677 | 1,708 |
| 9 | 1,589 | 1,486 | 1,510 |
| 10 | 1,285 | 1,321 | 1,269 |
| 11 | 1,264 | 1,211 | 1,218 |
| 12 | 1,272 | 1,220 | 1,246 |
| 13 | 1,315 | 1,288 | 1,283 |
| 14 | 1,419 | 1,330 | 1,304 |
| 15 | 1,523 | 1,492 | 1,523 |
| 16 | 1,866 | 1,652 | 1,579 |
| 17 | 2,183 | 2,221 | 2,007 |
| 18 | $\mathbf{2 , 5 8 1}$ | 1,504 | $\mathbf{2 , 3 2 9}$ |
| 19 | 1,517 | $\mathbf{2 , 2 9 6}$ | 1,608 |
| 20 | 1,159 | 1,134 | 1,204 |
| 21 | 952 | 1,013 | 1,018 |
| 22 | 676 | 767 | 766 |
| 23 | 612 | 508 | 488 |
| 24 | 321 | 310 | 323 |
| Total | $\mathbf{2 5 , 9 7 9}$ | $\mathbf{2 5 , 1 0 3}$ | $\mathbf{2 5 , 0 2 2}$ |
|  |  |  |  |

$\square$ Designates peak hour

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Table 2: 15-Minute Counts to Identify PM Peak Hour

| Ending <br> Time | Tues <br> $\mathbf{5 / 8 / 2 0 1 8}$ | Wed <br> $\mathbf{5 / 9 / 2 0 1 8}$ | Thurs <br> $\mathbf{5 / 1 0 / 2 0 1 8}$ |
| :---: | :---: | :---: | :---: |
| $16: 45$ | 469 | 567 | 510 |
| $17: 00$ | 539 | 540 | 508 |
| $17: 15$ | 535 | 429 | 522 |
| $17: 30$ | 674 | 382 | $\mathbf{6 5 1}$ |
| $17: 45$ | 722 | 258 | $\mathbf{6 0 2}$ |
| $18: 00$ | 650 | 435 | 554 |
| $18: 15$ | 407 | 637 | 415 |
| $18: 30$ | 402 | 544 | 422 |
| $18: 45$ | 368 | 584 | 392 |
| $19: 00$ | 340 | 531 | 379 |
| $19: 15$ | 339 | 328 | 312 |
| PM Pk Hr | $\mathbf{2 , 5 8 1}$ | $\mathbf{2 , 2 9 6}$ | $\mathbf{2 , 3 2 9}$ |
| Percent | $9.9 \%$ | $9.1 \%$ | $9.3 \%$ |
| PHF | 0.89 | 0.90 | 0.89 |

Figure 1 illustrates the hourly traffic counts collected on Neptune Road, east of Partin Settlement Road on Tuesday, May 8, 2018. This was the highest count measured within the project limits and is typical of the traffic patterns on Neptune Road. In addition to the PM peak, a second peak was observed during the morning hours; however, the volumes are significantly lower than measured in the PM peak hour. This is a very normal occurrence in urban areas as the PM peak hour is typically higher than the AM peak hour. As summarized in Table 3, the morning peak hour accounted for between 6.7 percent and 6.9 percent of the daily traffic volume. The peak hour factors ranged from 0.89 to 0.91 , suggesting no peak spreading.

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Figure 1: Hourly Traffic Volumes (Tuesday, May 8, 2018)


Table 3: 15-Minute Counts to Identify AM Peak Hour

| Ending <br> Time | Tues <br> $\mathbf{5 / 8 / 2 0 1 8}$ | Wed <br> $\mathbf{5 / 9 / 2 0 1 8}$ | Thurs <br> $\mathbf{5 / 1 0 / 2 0 1 8}$ |
| :---: | :---: | :---: | :---: |
| $6: 15$ | 298 | 278 | 281 |
| $6: 30$ | 358 | 339 | 294 |
| $6: 45$ | 396 | 377 | 370 |
| $7: 00$ | 426 | 442 | 427 |
| $7: 15$ | 414 | $\mathbf{3 8 3}$ | 395 |
| $7: 30$ | 359 | 382 | 372 |
| $7: 45$ | 468 | 449 | 460 |
| $8: 00$ | 487 | 463 | 481 |
| $8: 15$ | 397 | 355 | 407 |
| $8: 30$ | 453 | 397 | 346 |
| $8: 45$ | 392 | 380 | 378 |
| 9:00 | 347 | 354 | 379 |
| AM Pk Hr | $\mathbf{1 , 8 0 5}$ | $\mathbf{1 , 6 7 7}$ | $\mathbf{1 , 7 0 8}$ |
| Percent | $6.9 \%$ | $6.7 \%$ | $6.8 \%$ |
| PHF | 0.93 | 0.91 | 0.89 |

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Kimley-Horn proposes to use the standard $K$ for developing future PM peak hour turning movements. However; we propose to develop the AM peak hour turning movements by applying $7.0 \%$ to the projected AADT.

Minimal guidance is provided by FDOT for developing the morning peak hour turning movements. Section 4.4.5 Develop Project Traffic Forecast in Detail, of the Project Traffic Forecasting Handbook (January 2014) provides the following:
4. Use $K$ and $D$ factors to develop directional design hour traffic projections in the peak periods. AM and PM forecasts usually involve reversing the peak direction of flow.
5. Review the AM and PM design hour volumes for consistency with the trip generation activity pattern of the projected land uses in the vicinity and adjust if necessary. Such adjustments are made with reference to observed differences in travel characteristics such as numbers of trips and directional splits that occur during morning and evening peak periods. Directional traffic counts collected at local land use sites may provide the necessary data or the ITE Trip Generation Manual may be used to obtain the peak period trip generation characteristics of various land use/special generator sites.

As noted above, while point 4 may suggest (no specific direction is provided) reversing the PM forecasts for the AM forecast, point 5 directs the manual adjustments for reasonableness. Based on the characteristics of Neptune Road traffic (and consistent with typical conditions for urban roadways) the AM peak hour traffic is significantly lower than the PM peak hour traffic. Therefore, we believe the use of 7.0 percent of the AADT to develop the AM peak hour turning movement volumes is appropriate and consistent with the guidance provided in point 5 above, which recognizes the need to adjust the project volumes based on specific characteristics of the corridor.

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## Attachment A

72-Hour Traffic Count Neptune Road, East of Partin Settlement Road

# Roadway Count Summary 

Start Date 05/08/2018 (Tue)
Start Time 00:00
Stop Date 05/10/2018 (Thu)
Stop Time 24:00
County Osceola
Location 1A:Neptune Rd: SE of Partin Settlement Rd



| 8-May-18 (Tue) | TOTAL TWO WAY (Northbound Volume + Southbound Volume) |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 00 | 01 | 02 | 03 | 04 | 05 | 06 | 07 | 08 | 09 | 10 | 11 |
| 15 | 65 | 30 | 29 | 21 | 36 | 75 | 298 | 414 | 397 | 341 | 323 | 287 |
| 30 | 54 | 29 | 24 | 21 | 46 | 120 | 358 | 359 | 453 | 290 | 321 | 361 |
| 45 | 54 | 20 | 22 | 24 | 43 | 158 | 396 | 468 | 392 | 326 | 307 | 344 |
| 00 | 41 | 21 | 16 | 21 | 84 | 185 | 426 | 487 | 347 | 328 | 313 | 280 |
| Hr Total | 214 | 100 | 91 | 87 | 209 | 538 | 1478 | 1728 | 1589 | 1285 | 1264 | 1272 |
| End Time | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 |
| 15 | 313 | 343 | 376 | 449 | 623 | 535 | 407 | 339 | 217 | 168 | 254 | 92 |
| 30 | 331 | 356 | 380 | 497 | 552 | 674 | 402 | 296 | 270 | 190 | 132 | 81 |
| 45 | 351 | 336 | 373 | 481 | 469 | 722 | 368 | 283 | 236 | 173 | 89 | 82 |
| 00 | 320 | 384 | 394 | 439 | 539 | 650 | 340 | 241 | 229 | 145 | 137 | 66 |
| Hr Total | 1315 | 1419 | 1523 | 1866 | 2183 | 2581 | 1517 | 1159 | 952 | 676 | 612 | 321 |
| 24 | r Total: | 25,979 |  |  |  | Peak Ho | Analysis | Begins | Volume | Pk Hr Fac |  |  |
|  | -00-8:00 | 1,728 |  | 00-5:00 | 2,183 | AM | k Hour: | 7:30 | 1,805 | 0.93 |  |  |
|  | 00-9:00 | 1,589 |  | 00-6:00 | 2,581 | PM | k Hour: | 17:00 | 2,581 | 0.89 |  |  |

# Roadway Count Summary 

Start Date 05/08/2018 (Tue)
Start Time 00:00
Stop Date 05/10/2018 (Thu)
Stop Time 24:00
County Osceola
Location 1A:Neptune Rd: SE of Partin Settlement Rd

| 9-May-18 (Wed) | Northbound Volume |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 00 | 01 | 02 | 03 | 04 | 05 | 06 | 07 | 08 | 09 | 10 | 11 |
| 15 | 17 | 5 | 9 | 6 | 22 | 62 | 169 | 238 | 203 | 180 | 158 | 141 |
| 30 | 14 | 2 | 9 | 19 | 35 | 82 | 221 | 170 | 260 | 197 | 149 | 132 |
| 45 | 15 | 10 | 10 | 11 | 42 | 81 | 205 | 278 | 197 | 194 | 156 | 132 |
| 00 | 8 | 8 | 6 | 12 | 34 | 123 | 242 | 306 | 199 | 174 | 141 | 158 |
| Hr Total | 54 | 25 | 34 | 48 | 133 | 348 | 837 | 992 | 859 | 745 | 604 | 563 |
| End Time | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 |
| 15 | 140 | 139 | 136 | 157 | 220 | 123 | 162 | 119 | 97 | 71 | 56 | 38 |
| 30 | 156 | 132 | 160 | 174 | 187 | 100 | 153 | 108 | 112 | 71 | 41 | 34 |
| 45 | 139 | 167 | 133 | 148 | 207 | 50 | 167 | 83 | 102 | 53 | 33 | 22 |
| 00 | 162 | 127 | 190 | 142 | 195 | 119 | 141 | 92 | 86 | 53 | 36 | 20 |
| Hr Total | 597 | 565 | 619 | 621 | 809 | 392 | 623 | 402 | 397 | 248 | 166 | 114 |
| 24 Hour Total: 10,795 |  |  |  |  |  | Peak H | Analysis | Begins | Volume | Pk Hr Fac |  |  |
| AM 7:00-8:00 |  | 992 |  | 0-5:00 | 809 | AM | Hour: | 7:00 | 992 | 0.81 |  |  |
| AM 8:00-9:00 |  | 859 |  | 0-6:00 | 392 | PM | k Hour: | 16:00 | 809 | 0.92 |  |  |


| 9-May-18 (Wed) | Southbound Volume |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 00 | 01 | 02 | 03 | 04 | 05 | 06 | 07 | 08 | 09 | 10 | 11 |
| 15 | 42 | 14 | 14 | 15 | 15 | 23 | 109 | 145 | 152 | 123 | 154 | 163 |
| 30 | 37 | 19 | 16 | 16 | 18 | 43 | 118 | 212 | 137 | 115 | 132 | 168 |
| 45 | 37 | 20 | 19 | 14 | 22 | 53 | 172 | 171 | 183 | 168 | 155 | 166 |
| 00 | 29 | 13 | 20 | 9 | 23 | 64 | 200 | 157 | 155 | 170 | 166 | 160 |
| Hr Total | 145 | 66 | 69 | 54 | 78 | 183 | 599 | 685 | 627 | 576 | 607 | 657 |
| End Time | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 |
| 15 | 178 | 151 | 214 | 242 | 334 | 306 | 475 | 209 | 143 | 152 | 95 | 55 |
| 30 | 170 | 202 | 221 | 255 | 373 | 282 | 391 | 202 | 155 | 127 | 89 | 51 |
| 45 | 181 | 193 | 235 | 260 | 360 | 208 | 417 | 162 | 161 | 116 | 88 | 43 |
| 00 | 162 | 219 | 203 | 274 | 345 | 316 | 390 | 159 | 157 | 124 | 70 | 47 |
| Hr Total | 691 | 765 | 873 | 1031 | 1412 | 1112 | 1673 | 732 | 616 | 519 | 342 | 196 |
| 24 Hour Total: 14,308 |  |  |  |  |  | Peak Hour | Analysis | Begins | Volume | Pk Hr Fac |  |  |
| AM 7:00-8:00 |  | 685 |  | 00-5:00 | 1,412 | AM | k Hour: | 7:00 | 685 | 0.81 |  |  |
| AM 8:00-9:00 |  | 627 |  | 00-6:00 | 1,112 | PM | k Hour: | 18:00 | 1,673 | 0.88 |  |  |


| 9-May-18 (Wed) | TOTAL TWO WAY (Northbound Volume + Southbound Volume) |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 00 | 01 | 02 | 03 | 04 | 05 | 06 | 07 | 08 | 09 | 10 | 11 |
| 15 | 59 | 19 | 23 | 21 | 37 | 85 | 278 | 383 | 355 | 303 | 312 | 304 |
| 30 | 51 | 21 | 25 | 35 | 53 | 125 | 339 | 382 | 397 | 312 | 281 | 300 |
| 45 | 52 | 30 | 29 | 25 | 64 | 134 | 377 | 449 | 380 | 362 | 311 | 298 |
| 00 | 37 | 21 | 26 | 21 | 57 | 187 | 442 | 463 | 354 | 344 | 307 | 318 |
| Hr Total | 199 | 91 | 103 | 102 | 211 | 531 | 1436 | 1677 | 1486 | 1321 | 1211 | 1220 |
| End Time | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 |
| 15 | 318 | 290 | 350 | 399 | 554 | 429 | 637 | 328 | 240 | 223 | 151 | 93 |
| 30 | 326 | 334 | 381 | 429 | 560 | 382 | 544 | 310 | 267 | 198 | 130 | 85 |
| 45 | 320 | 360 | 368 | 408 | 567 | 258 | 584 | 245 | 263 | 169 | 121 | 65 |
| 00 | 324 | 346 | 393 | 416 | 540 | 435 | 531 | 251 | 243 | 177 | 106 | 67 |
| Hr Total | 1288 | 1330 | 1492 | 1652 | 2221 | 1504 | 2296 | 1134 | 1013 | 767 | 508 | 310 |
| 24 H | r Total: | 25,103 |  |  |  | Peak Hour | Analysis | Begins | Volume | Pk Hr Fac |  |  |
| AM | -00-8:00 | 1,677 |  | :00-5:00 | 2,221 | AM | k Hour: | 7:00 | 1,677 | 0.91 |  |  |
| AM | 00-9:00 | 1,486 |  | :00-6:00 | 1,504 | PM | k Hour: | 18:00 | 2,296 | 0.90 |  |  |

# Roadway Count Summary 

Start Date 05/08/2018 (Tue)
Start Time 00:00
Stop Date 05/10/2018 (Thu)
Stop Time 24:00
County Osceola
Location 1A:Neptune Rd: SE of Partin Settlement Rd

| 10-May-18 (Thu) | Northbound Volume |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 00 | 01 | 02 | 03 | 04 | 05 | 06 | 07 | 08 | 09 | 10 | 11 |
| 15 | 24 | 19 | 6 | 10 | 33 | 60 | 164 | 221 | 243 | 171 | 141 | 121 |
| 30 | 14 | 8 | 10 | 21 | 24 | 78 | 195 | 179 | 199 | 177 | 152 | 150 |
| 45 | 16 | 11 | 12 | 11 | 41 | 80 | 208 | 294 | 214 | 163 | 124 | 140 |
| 00 | 9 | 9 | 9 | 10 | 46 | 114 | 223 | 313 | 218 | 187 | 170 | 145 |
| Hr Total | 63 | 47 | 37 | 52 | 144 | 332 | 790 | 1007 | 874 | 698 | 587 | 556 |
| End Time | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 |
| 15 | 141 | 161 | 135 | 39 | 176 | 173 | 153 | 104 | 91 | 85 | 49 | 39 |
| 30 | 152 | 155 | 179 | 40 | 168 | 172 | 159 | 123 | 110 | 67 | 54 | 26 |
| 45 | 147 | 144 | 142 | 236 | 189 | 177 | 150 | 109 | 93 | 68 | 39 | 25 |
| 00 | 143 | 155 | 15 | 221 | 159 | 157 | 159 | 96 | 111 | 54 | 28 | 16 |
| Hr Total | 583 | 615 | 471 | 536 | 692 | 679 | 621 | 432 | 405 | 274 | 170 | 106 |
| 24 | r Total: | 10,771 |  |  |  | Peak | Analysis | Begins | Volume | Pk Hr Fac |  |  |
| AM | -00-8:00 | 1,007 |  | 0-5:00 | 692 | AM | k Hour: | 7:00 | 1,007 | 0.80 |  |  |
|  | 00-9:00 | 874 |  | 00-6:00 | 679 |  | k Hour: | 16:00 | 692 | 0.92 |  |  |


| 10-May-18 (Thu) | Southbound Volume |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 00 | 01 | 02 | 03 | 04 | 05 | 06 | 07 | 08 | 09 | 10 | 11 |
| 15 | 46 | 24 | 14 | 11 | 14 | 21 | 117 | 174 | 164 | 155 | 146 | 186 |
| 30 | 39 | 23 | 15 | 11 | 11 | 44 | 99 | 193 | 147 | 139 | 173 | 175 |
| 45 | 34 | 23 | 12 | 13 | 14 | 57 | 162 | 166 | 164 | 126 | 165 | 180 |
| 00 | 33 | 17 | 10 | 20 | 30 | 56 | 204 | 168 | 161 | 151 | 147 | 149 |
| Hr Total | 152 | 87 | 51 | 55 | 69 | 178 | 582 | 701 | 636 | 571 | 631 | 690 |
| End Time | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 |
| 15 | 175 | 178 | 219 | 281 | 334 | 349 | 262 | 208 | 170 | 110 | 73 | 49 |
| 30 | 168 | 178 | 287 | 248 | 311 | 479 | 263 | 222 | 168 | 124 | 97 | 59 |
| 45 | 171 | 168 | 320 | 245 | 321 | 425 | 242 | 187 | 157 | 136 | 81 | 53 |
| 00 | 186 | 165 | 226 | 269 | 349 | 397 | 220 | 155 | 118 | 122 | 67 | 56 |
| Hr Total | 700 | 689 | 1052 | 1043 | 1315 | 1650 | 987 | 772 | 613 | 492 | 318 | 217 |
| 24 Hour Total: 14,251 |  |  |  |  |  | Peak Hour Analysis |  | Begins | Volume | Pk Hr Fac |  |  |
| AM 7:00-8:00 |  | 701 | PM 4:00-5:00 |  | 1,315 | AM Peak Hour: |  | 7:00 | 701 | 0.91 |  |  |
| AM 8:00-9:00 |  | 636 | PM 5:00-6:00 |  | 1,650 | PM Peak Hour: |  | 17:00 | 1,650 | 0.86 |  |  |


| 10-May-18 (Thu) | TOTAL TWO WAY (Northbound Volume + Southbound Volume) |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 00 | 01 | 02 | 03 | 04 | 05 | 06 | 07 | 08 | 09 | 10 | 11 |
| 15 | 70 | 43 | 20 | 21 | 47 | 81 | 281 | 395 | 407 | 326 | 287 | 307 |
| 30 | 53 | 31 | 25 | 32 | 35 | 122 | 294 | 372 | 346 | 316 | 325 | 325 |
| 45 | 50 | 34 | 24 | 24 | 55 | 137 | 370 | 460 | 378 | 289 | 289 | 320 |
| 00 | 42 | 26 | 19 | 30 | 76 | 170 | 427 | 481 | 379 | 338 | 317 | 294 |
| Hr Total | 215 | 134 | 88 | 107 | 213 | 510 | 1372 | 1708 | 1510 | 1269 | 1218 | 1246 |
| End Time | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 |
| 15 | 316 | 339 | 354 | 320 | 510 | 522 | 415 | 312 | 261 | 195 | 122 | 88 |
| 30 | 320 | 333 | 466 | 288 | 479 | 651 | 422 | 345 | 278 | 191 | 151 | 85 |
| 45 | 318 | 312 | 462 | 481 | 510 | 602 | 392 | 296 | 250 | 204 | 120 | 78 |
| 00 | 329 | 320 | 241 | 490 | 508 | 554 | 379 | 251 | 229 | 176 | 95 | 72 |
| Hr Total | 1283 | 1304 | 1523 | 1579 | 2007 | 2329 | 1608 | 1204 | 1018 | 766 | 488 | 323 |
| 24 H | r Total: | 25,022 |  |  |  | Peak Hoar | Analysis | Begins | Volume | Pk Hr Fac |  |  |
| AM | -00-8:00 | 1,708 |  | :00-5:00 | 2,007 | AM | k Hour: | 7:00 | 1,708 | 0.89 |  |  |
| AM | 00-9:00 | 1,510 |  | :00-6:00 | 2,329 | PM | k Hour: | 17:00 | 2,329 | 0.89 |  |  |

## Roadway Count Summary

Average

Start Date 05/08/2018 (Tue)
Stop Date 05/10/2018 (Thu) County Osceola
Location 1A:Neptune Rd: SE of Partin Settlement Rd

Start Time 00:00
Stop Time 24:00

| 3 Avg | Northbound Volume |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 00 | 01 | 02 | 03 | 04 | 05 | 06 | 07 | 08 | 09 | 10 | 11 |
| 15 | 19 | 11 | 10 | 9 | 25 | 57 | 169 | 230 | 232 | 176 | 156 | 132 |
| 30 | 14 | 6 | 9 | 17 | 30 | 80 | 211 | 172 | 249 | 175 | 159 | 149 |
| 45 | 14 | 9 | 11 | 12 | 38 | 79 | 210 | 284 | 210 | 178 | 145 | 141 |
| 00 | 7 | 8 | 8 | 12 | 42 | 118 | 230 | 314 | 205 | 179 | 152 | 142 |
| Hr Total | 54 | 34 | 38 | 50 | 135 | 334 | 820 | 1000 | 896 | 708 | 612 | 564 |
| End Time | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 |
| 15 | 139 | 147 | 142 | 120 | 218 | 154 | 150 | 120 | 91 | 77 | 77 | 37 |
| 30 | 145 | 150 | 164 | 134 | 178 | 144 | 148 | 112 | 111 | 72 | 49 | 31 |
| 45 | 148 | 156 | 141 | 183 | 188 | 138 | 151 | 97 | 96 | 63 | 33 | 24 |
| 00 | 149 | 147 | 133 | 170 | 176 | 146 | 138 | 93 | 99 | 50 | 37 | 18 |
| Hr Total | 581 | 600 | 580 | 607 | 760 | 582 | 587 | 422 | 397 | 262 | 196 | 110 |
| 24 Hour Total: 10,929 |  |  |  |  |  | Peak Hour Analysis |  | Begins | Volume | Pk Hr Fac |  |  |
| AM 7:00-8:00 |  | 1,000 | PM 4:00-5:00 |  | 760 | AM Peak Hour: |  | 7:00 | 1,000 | 0.80 |  |  |
| AM 8:00-9:00 |  | 896 | PM 5:00-6:00 |  | 582 | PM Peak Hour: |  | 16:00 | 760 | 0.87 |  |  |


| 3 Avg | Southbound Volume |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 00 | 01 | 02 | 03 | 04 | 05 | 06 | 07 | 08 | 09 | 10 | 11 |
| 15 | 46 | 20 | 14 | 12 | 15 | 23 | 116 | 168 | 155 | 147 | 151 | 168 |
| 30 | 39 | 21 | 16 | 12 | 15 | 43 | 119 | 199 | 150 | 131 | 150 | 180 |
| 45 | 38 | 19 | 14 | 12 | 16 | 64 | 171 | 175 | 174 | 147 | 157 | 179 |
| 00 | 33 | 15 | 13 | 12 | 30 | 63 | 202 | 163 | 155 | 158 | 160 | 156 |
| Hr Total | 156 | 75 | 57 | 48 | 76 | 193 | 608 | 705 | 634 | 583 | 618 | 683 |
| End Time | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 |
| 15 | 177 | 177 | 218 | 269 | 344 | 341 | 336 | 206 | 148 | 118 | 99 | 54 |
| 30 | 181 | 191 | 245 | 271 | 352 | 425 | 308 | 205 | 160 | 121 | 89 | 53 |
| 45 | 182 | 180 | 260 | 273 | 327 | 389 | 297 | 177 | 153 | 119 | 77 | 51 |
| 00 | 175 | 203 | 209 | 278 | 353 | 400 | 278 | 155 | 134 | 116 | 76 | 50 |
| Hr Total | 715 | 751 | 932 | 1091 | 1376 | 1555 | 1219 | 743 | 595 | 474 | 341 | 208 |
| 24 Hour Total: 14,436 |  |  |  |  |  | Peak H | Analysis | Begins | Volume | Pk Hr Fac |  |  |
| AM 7:00-8:00 |  | 705 | PM 5:00-6:00 |  | 1,376 | AM Peak Hour: |  | 7:00 | 705 | 0.88 |  |  |
| AM 8:00-9:00 |  | 634 |  |  | 1,555 | PM Peak Hour: |  | 17:00 | 1,555 | 0.91 |  |  |


| 3 Avg | TOTAL TWO WAY (Northbound Volume + Southbound Volume) |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| End Time | 00 | 01 | 02 | 03 | 04 | 05 | 06 | 07 | 08 | 09 | 10 | 11 |
| 15 | 65 | 31 | 24 | 21 | 40 | 80 | 286 | 397 | 386 | 323 | 307 | 299 |
| 30 | 53 | 27 | 25 | 29 | 45 | 122 | 330 | 371 | 399 | 306 | 309 | 329 |
| 45 | 52 | 28 | 25 | 24 | 54 | 143 | 381 | 459 | 383 | 326 | 302 | 321 |
| 00 | 40 | 23 | 20 | 24 | 72 | 181 | 432 | 477 | 360 | 337 | 312 | 297 |
| Hr Total | 210 | 109 | 94 | 98 | 211 | 526 | 1429 | 1704 | 1528 | 1292 | 1230 | 1246 |
| End Time | 12 | 13 | 14 | 15 | 16 | 17 | 18 | 19 | 20 | 21 | 22 | 23 |
| 15 | 316 | 324 | 360 | 389 | 562 | 495 | 486 | 326 | 239 | 195 | 176 | 91 |
| 30 | 326 | 341 | 409 | 405 | 530 | 569 | 456 | 317 | 272 | 193 | 138 | 84 |
| 45 | 330 | 336 | 401 | 457 | 515 | 527 | 448 | 275 | 250 | 182 | 110 | 75 |
| 00 | 324 | 350 | 343 | 448 | 529 | 546 | 417 | 248 | 234 | 166 | 113 | 68 |
| Hr Total | 1296 | 1351 | 1513 | 1699 | 2136 | 2137 | 1807 | 1166 | 995 | 736 | 537 | 318 |
|  | r Total: | 25,368 |  |  |  | Peak Ho | Analysis | Begins | Volume | Pk Hr Fac |  |  |
|  | 700-8:00 | 1,704 |  | 00-5:00 | 2,136 | AM | k Hour: | 7:00 | 1,704 | 0.89 |  |  |
|  | 00-9:00 | 1,528 |  | 00-6:00 | 2,137 | PM | k Hour: | 17:00 | 2,138 | 0.94 |  |  |

